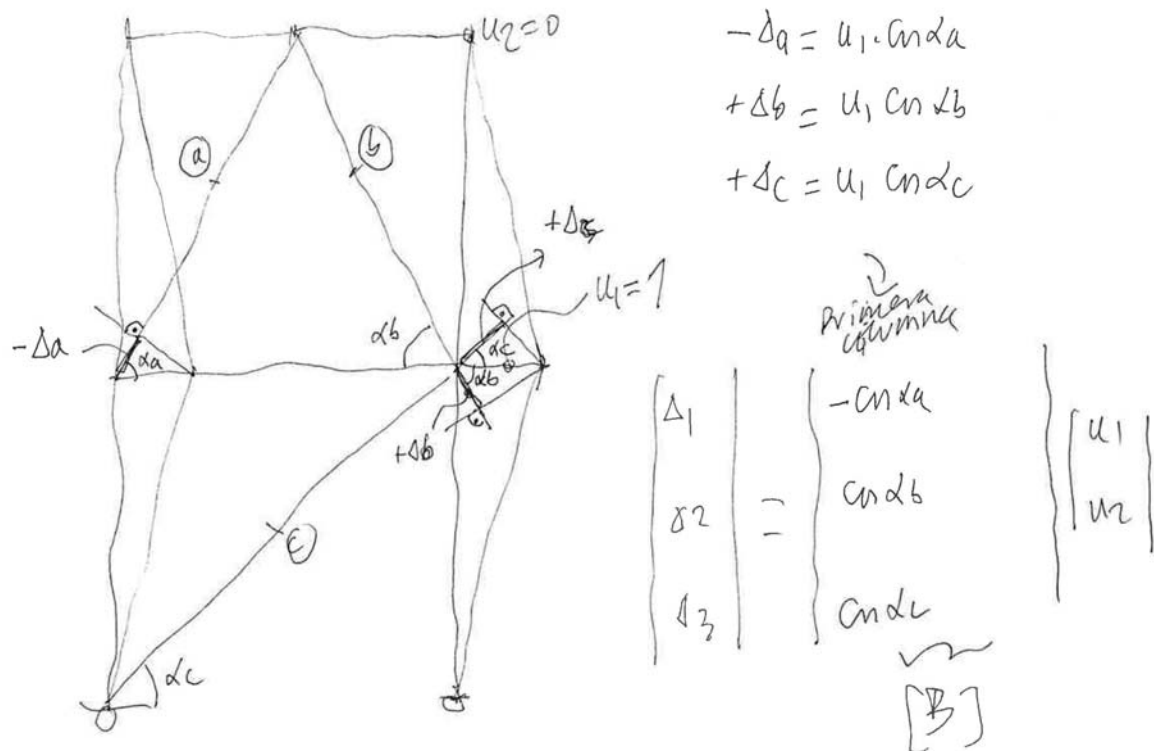


**ESTRUCTURAS I: EJERCICIOS DE
APLICACIÓN DEL MÉTODO BÁSICO DE LA
RIGIDEZ**



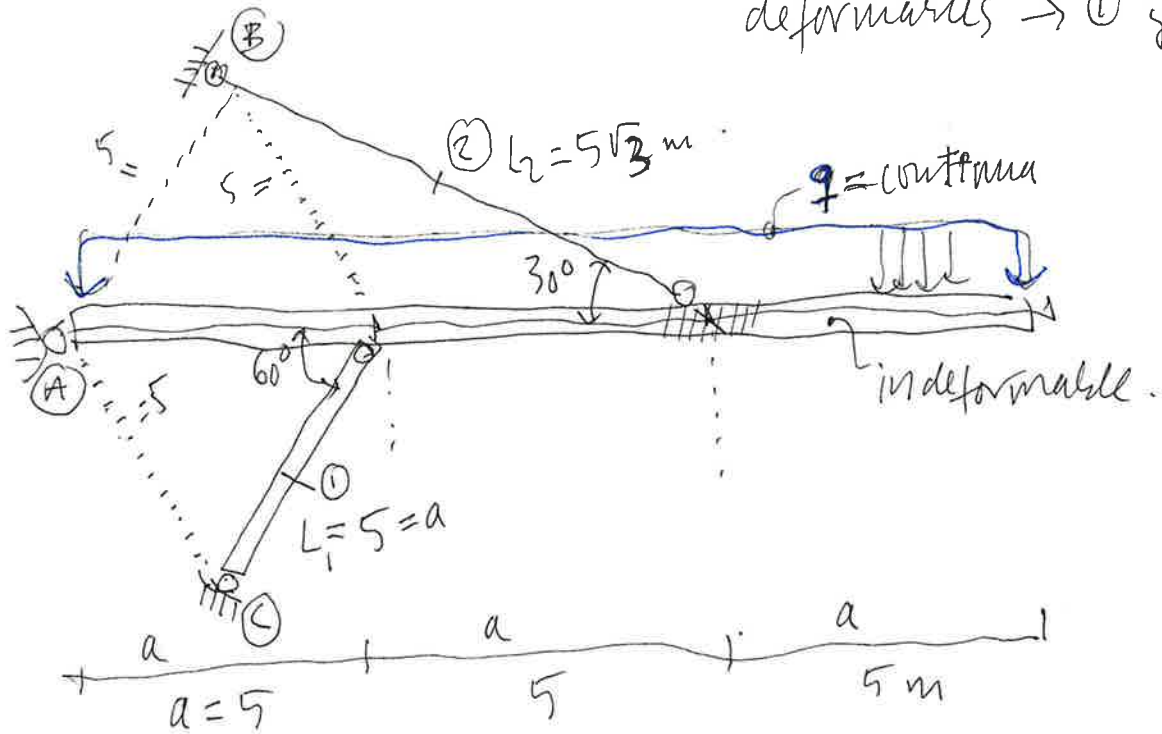
JOSÉ L. FERNÁNDEZ CABO

MADRID, Marzo 2012 (v1)

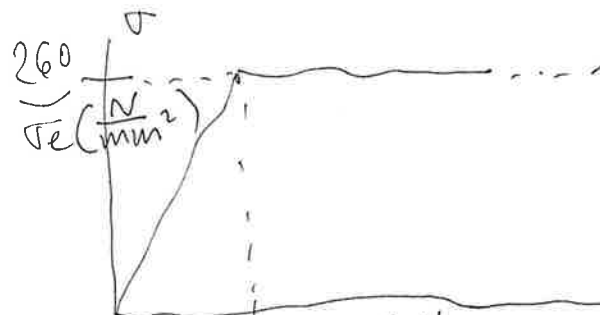


Reconocimiento - NoComercial - SinObraDerivada (by-nc-nd)

deformables \rightarrow ① y ②

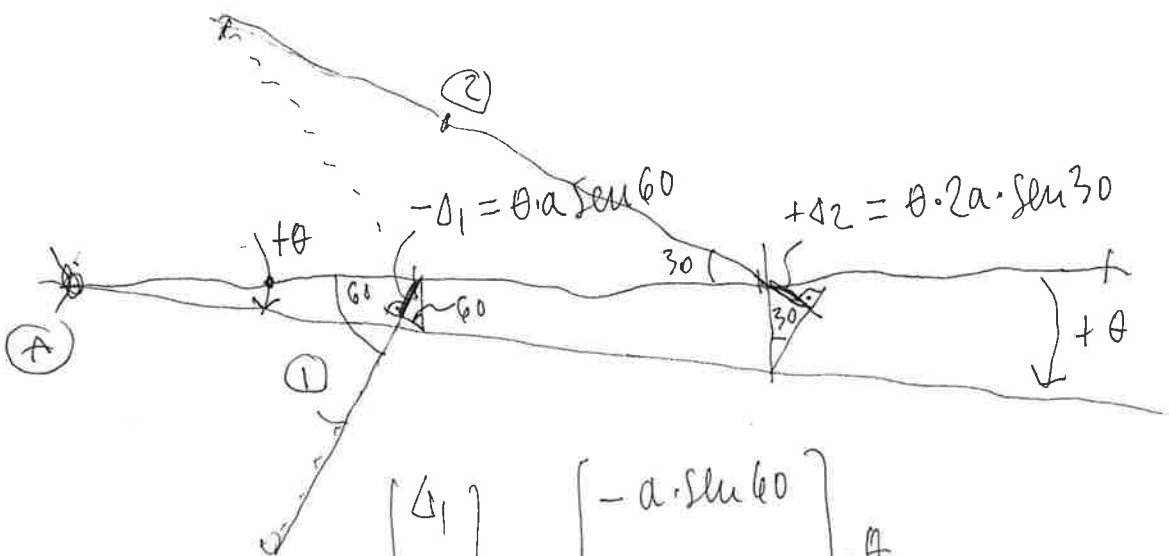


material
de ① y ②



$$\epsilon_e = 1.3\% \Rightarrow E = 200 \frac{\text{kn}}{\text{mm}^2}$$

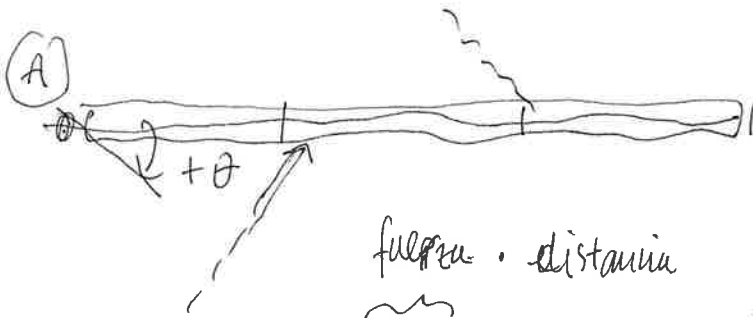
1 solo g.d.l; p.g. $\theta = \text{giro en } A$



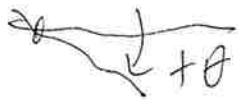
$$\begin{bmatrix} \Delta_1 \\ \Delta_2 \end{bmatrix} = \begin{bmatrix} -a \cdot \text{sen } 60 \\ 2a \cdot \text{sen } 30 \end{bmatrix} \cdot \theta$$

$$\{\Delta\} = [B] \cdot \{\delta\}$$

$\{P\}$ asociado a θ



$$\downarrow M = (q \cdot 3a) \cdot \frac{3a}{2} = q \frac{9a^2}{2} = \{P\} \quad \{\delta\} = \theta$$



$$[K] = [B]^T \cdot [K_M] \cdot [B] =$$

$$[K] = \begin{bmatrix} -a \sin 60 & 2a \sin 30 \end{bmatrix} \begin{bmatrix} k_1 & 0 \\ 0 & k_2 \end{bmatrix} \begin{bmatrix} -a \sin 60 \\ 2a \sin 30 \end{bmatrix}$$

$$[K] = a^2 \sin^2 60 \cdot k_1 + 4a^2 \sin^2 30 \cdot k_2$$

ojo unidades, depende de $\{P\}$ y $\{\delta\}$

$$\Rightarrow \{P\} = [K] \cdot \{\delta\}$$

$$\begin{matrix} \text{KN} \cdot \text{mm} & \begin{matrix} \text{rad} \\ \text{KN} \cdot \text{mm} \end{matrix} \end{matrix}$$

es conveniente calcular k_1 y k_2 por separado antes de operar en $[K]$, ya que además aparecen en valores más veloz.

zona	$E \left(\frac{kN}{mm^2} \right)$	Area (mm^2) $\phi 35 mm$	$L (mm)$	$k_i = \left(\frac{EA}{L} \right)_i \left(\frac{kN}{mm} \right)$
①	200	962	$5 \cdot 10^3$	$38,48 \neq k_1$
②	200	962	$5\sqrt{3} \cdot 10^3$	$22,21 \neq k_2$

$$[K] = 5000^2 \cdot \sin^2 60 \cdot 38,48 + 4 \cdot 5000^2 \cdot \sin^2 30 \cdot 22,21 = 1,276 \cdot 10^9$$

$kN \cdot mm$

$$\{ P \} = [K] \cdot \{ \theta \}$$

$$q \cdot q \cdot \frac{5000^2}{2} = 1,276 \cdot 10^9 \cdot \frac{\theta}{rad}$$

en función de q

$$\theta = 0,088 \cdot q \text{ rad}$$

$\frac{kN}{mm}$

$$\begin{bmatrix} \Delta_1 \\ \Delta_2 \end{bmatrix} = \begin{bmatrix} -a \sin 60 \\ 2a \sin 30 \end{bmatrix} \cdot \theta = \begin{bmatrix} -5000 \cdot \sin 60 \cdot 0,088 \cdot q \left(\frac{kN}{mm} \right) \\ + 2 \cdot 5000 \cdot \sin 30 \cdot 0,088 \cdot q \left(\frac{kN}{mm} \right) \end{bmatrix} mm$$

$$\epsilon_{max} = \epsilon_e = 1,3 \cdot 10^{-3}$$

¿valor de q para que $\epsilon_{max} = \epsilon_e = 1,3 \cdot 10^{-3}$?

$$\Delta_{1,max} = 1,3 \cdot 10^{-3} \cdot 5 \cdot \sqrt{3} \cdot 10^3 mm = 11,26 mm$$

$$\Delta_{2,max} = 1,3 \cdot 10^{-3} \cdot 5 \cdot 10^3 mm = 6,5 mm$$

\Rightarrow

$$\begin{aligned} q &= 2,95 \cdot 10^{-2} \frac{kN}{mm} \\ q &= 1,477 \cdot 10^{-2} \frac{kN}{mm} \\ \downarrow \\ q &= 14,77 \frac{kN}{m} \end{aligned}$$

la más pequeña

en lo que sigue se toma $q = 10 \frac{\text{kv}}{\text{m}}$
 de modo que $(10 < 14,77)$ todos los cables están
 en régimen elástico.

de lo antes calculado

$$\{P\} = [K] \{ \theta \}$$

$$q = 10 \frac{\text{kv}}{\text{m}} = 10 \cdot 10^{-3} \frac{\text{kv}}{\text{mm}}$$

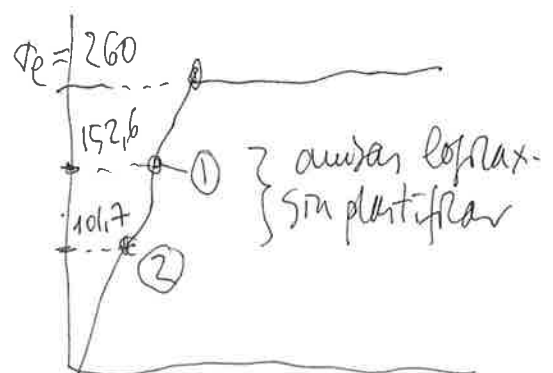
$$q \cdot \frac{9 \cdot 5000^2}{2} = 1,276 \cdot 10^9 \cdot \theta (\text{rad})$$

$$\frac{\text{kv}}{\text{mm}} \Rightarrow \theta = 8,81 \cdot 10^{-4} \text{ rad} \equiv 0,881 \text{ miliradianes} \equiv 0,881 \frac{\text{mm}}{\text{m}}$$

$$\begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix} = \begin{Bmatrix} -5000 \cdot \sin 60 \\ 2 \cdot 5000 \cdot \sin 30 \end{Bmatrix} \cdot \theta = \begin{Bmatrix} -3,815 \\ 4,406 \end{Bmatrix} \text{ mm}$$

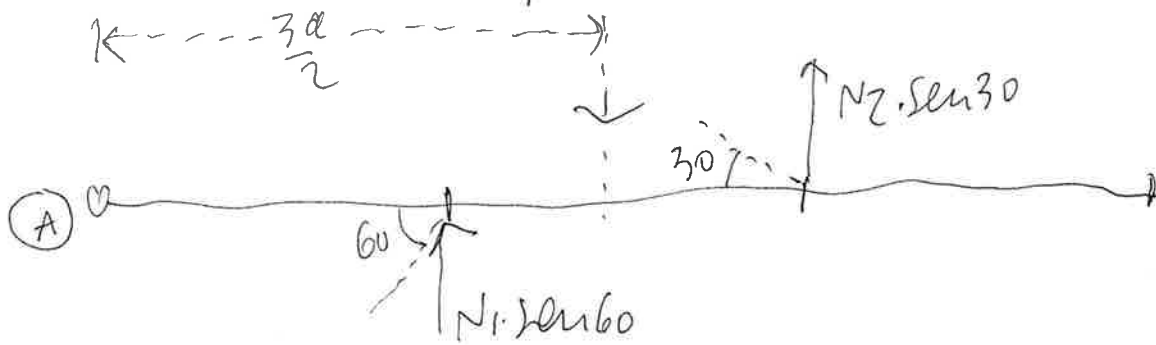
$$\begin{Bmatrix} N_1 \\ N_2 \end{Bmatrix} = \begin{Bmatrix} K_1 & 0 \\ 0 & K_2 \end{Bmatrix} \begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix} = \begin{Bmatrix} 38,48 & 0 \\ 0 & 22,21 \end{Bmatrix} \begin{Bmatrix} -3,815 \\ 4,406 \end{Bmatrix} = \begin{Bmatrix} -146,82 \\ 97,85 \end{Bmatrix} \text{ kN}$$

$$\begin{Bmatrix} \sigma_1 \\ \sigma_2 \end{Bmatrix} = \begin{Bmatrix} \frac{N_1}{A_1} \\ \frac{N_2}{A_2} \end{Bmatrix} = \begin{Bmatrix} \frac{146,82 \cdot 10^3}{962} \\ \frac{97,85 \cdot 10^3}{962} \end{Bmatrix} = \begin{Bmatrix} -152,6 \\ 101,72 \end{Bmatrix} \frac{\text{N}}{\text{mm}^2}$$



Comprobamos el equilibrio

$$P = q \cdot z \cdot a = 10 \cdot 3 \cdot 5 = \underline{150 \text{ kN}}$$



$$\sum M(A) = 0$$

$$\Rightarrow P \cdot \frac{150 \cdot 3}{2} = N_1 \sin 60 \cdot a + N_2 \sin 60 \cdot 2a$$

$$\frac{150 \cdot 3}{2} = 146,82 \cdot \sin 60 + 97,85 \cdot \sin 30 \cdot 2$$

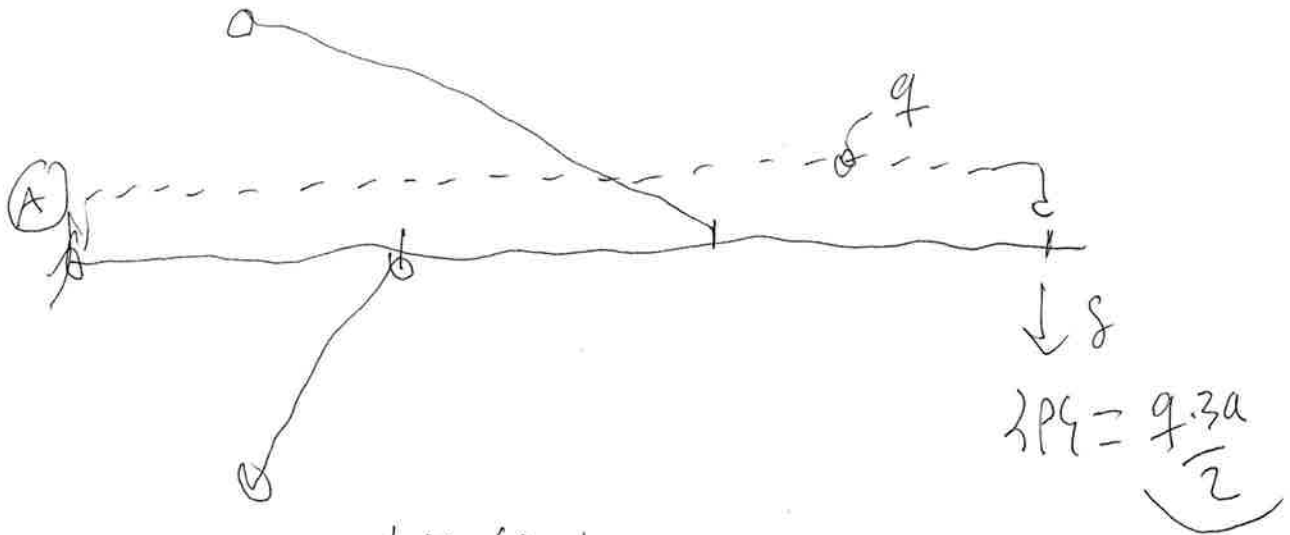
$$\underline{225}$$

$$\underline{224,9998}$$

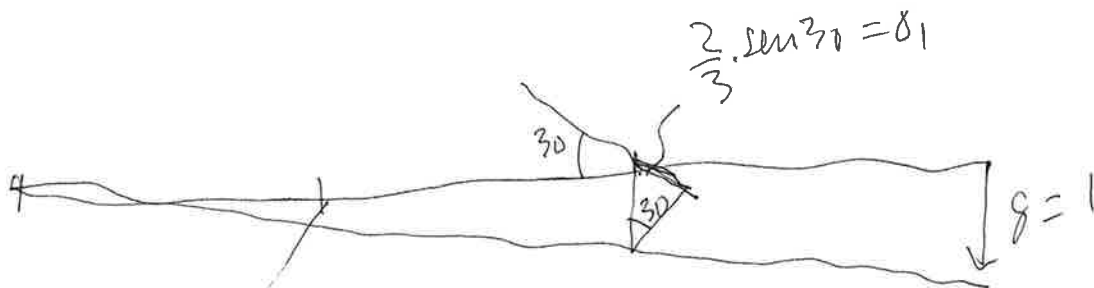
error de un dígito de decimal

OK

usamos otro g.d.l. : el descenso en punta



$$\begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix} = \begin{Bmatrix} -\frac{1}{3} \sin 60 \\ +\frac{2}{3} \sin 30 \end{Bmatrix} \cdot \delta$$



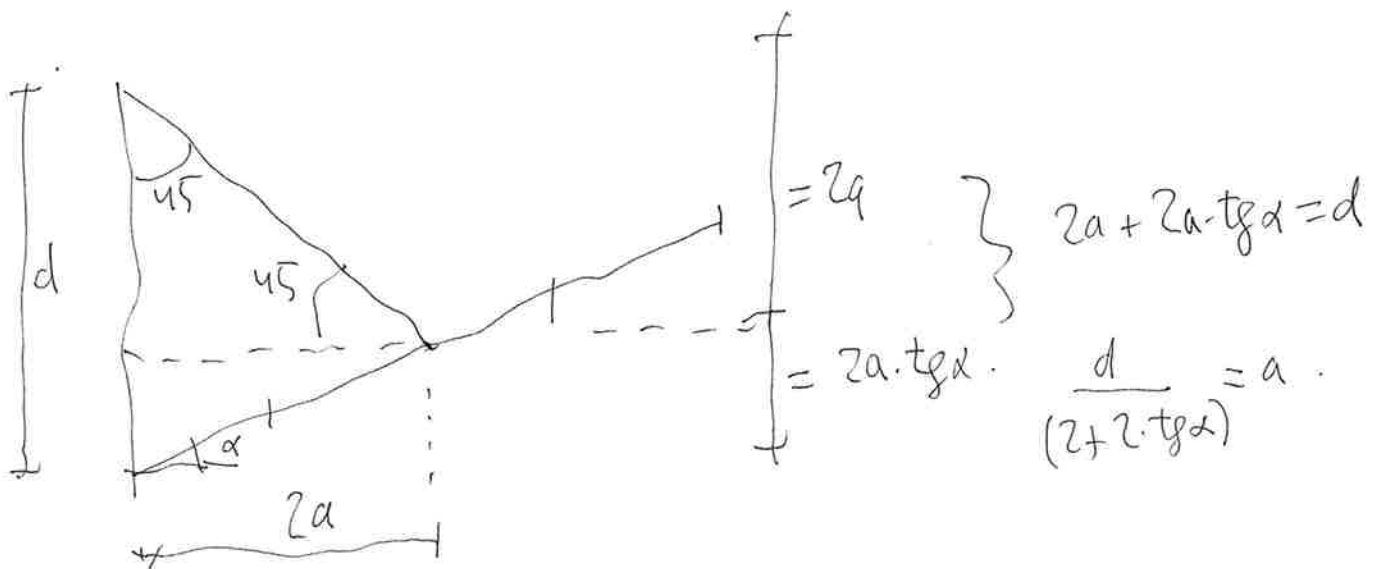
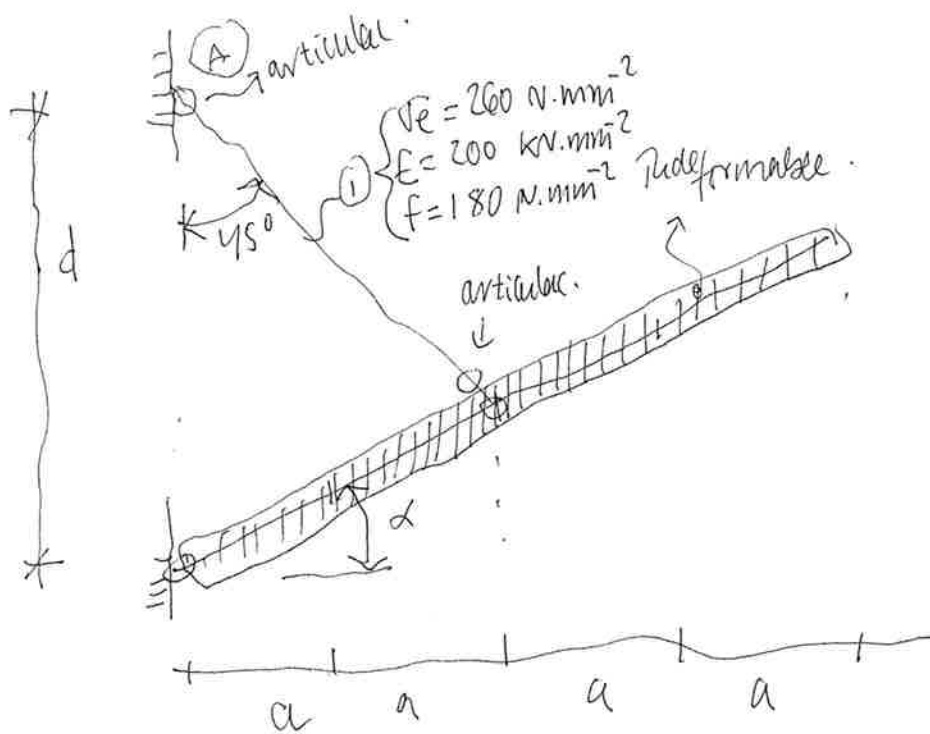
$$[B]^T \cdot [K] \cdot [B] = 5,67 \frac{\text{KN}}{\text{mm}}$$

$$\delta_4 = [K]^{-1} \cdot \delta P_4 = \frac{1}{5,67} \cdot \left(\frac{10 \cdot \frac{\text{KN}}{\text{m}} \cdot 3 \cdot 5 \text{ m}}{2} \right) = 13,23 \text{ mm}$$

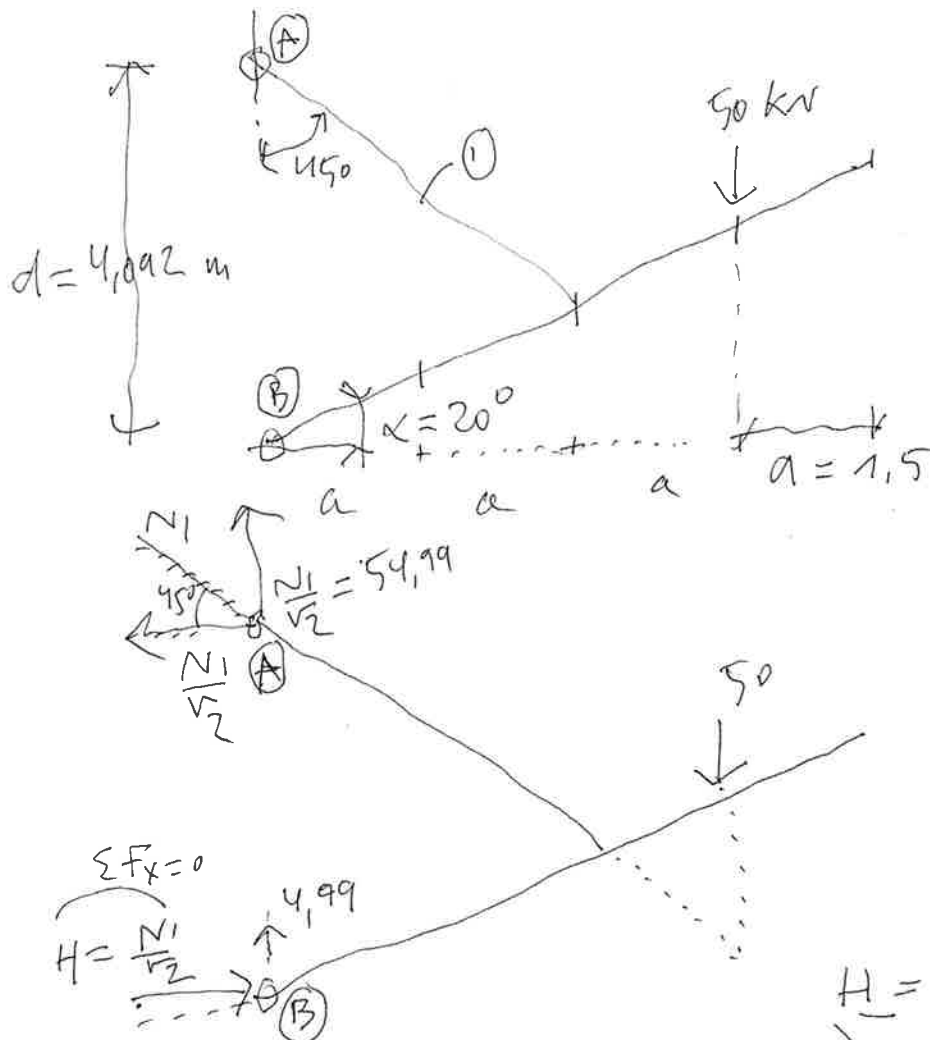
$$\begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix} = [B] \cdot \delta = \begin{Bmatrix} -3,819 \\ +4,41 \end{Bmatrix} \text{ mm}$$

igual que antes

OK. ... misma deflexión en redondeo.



Vamos a resolverlo para datos concretos:



$$\sum M(B) = 0 \quad \leftarrow +$$


$$\frac{N_1}{\sqrt{2}} \cdot d = 50.3a$$

$$H = \frac{N_1}{\sqrt{2}} = \frac{50 \cdot 3 \cdot 1,5}{4,092} = 54,99 \text{ kN}$$

$$\underline{N_1 = H \cdot \sqrt{2} = 77.76 \text{ kN}}$$

dimensionado del cable
para $f = 180$

$$A = \frac{N}{f} = \frac{77,76 \cdot 10^3 \text{ N}}{180 \frac{\text{N}}{\text{mm}^2}} = 432 \text{ mm}^2$$

Si ϕ $A = \frac{\pi \cdot \phi^2}{4} \Rightarrow \phi = \sqrt{\frac{4A}{\pi}} = 23,45 \approx 25 \text{ mm}$ 

$$\phi = 25 \text{ mm}$$

men $\phi 25 \Rightarrow A = \frac{\pi \cdot 25^2}{4} = 490,86 \text{ mm}^2$

$$A_{min} = 432 \text{ mm}$$

$$\tau_{\max} = f \cdot \frac{432}{490.89} = 158.4 \frac{\text{N}}{\text{mm}} \quad \dots \text{en el cable.}$$

• ¿cuanto se alarga el cable

$\sigma = 158,4 \frac{\text{N}}{\text{mm}^2} \rightarrow \underline{\underline{\epsilon = \frac{\sigma}{E} = \frac{158,4 \frac{\text{N}}{\text{mm}^2}}{200 \cdot 10^3 \frac{\text{N}}{\text{mm}^2}} = 0,792 \cdot 10^{-3}}}$

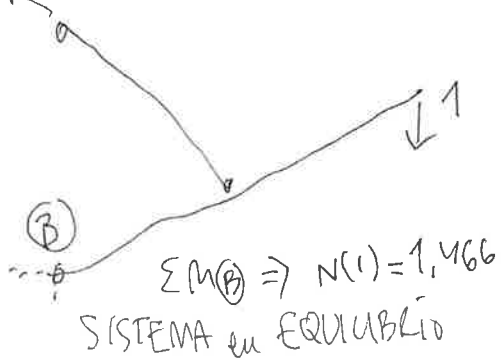
tambien $N = \left(\frac{EA}{L}\right) \cdot \Delta$

$$\Delta = \frac{N}{(EA/L)}$$

$\Delta = 3,36 \text{ mm}$

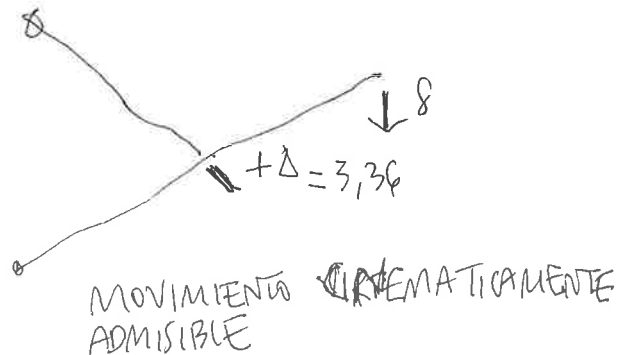
• ¿cuerpo desciende al extremo de la barra indeformable?

← NC(1) trabajos virtuales.

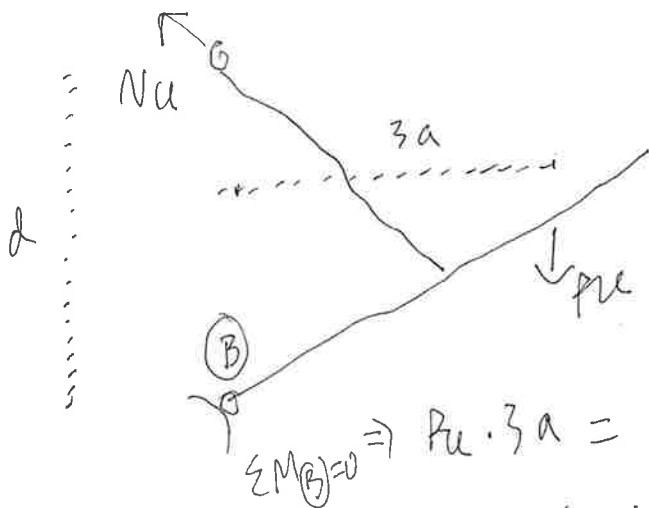


$$W_{EXT} = W_{INT}$$

$$1.8 = (+1,466) \cdot (+3,36) = \underline{4,92} \text{ mm}$$



carga última



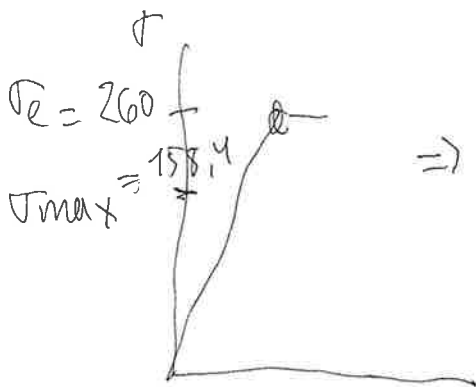
$$\begin{aligned} N_u &= \sigma_e \cdot A \\ &= 260 \cdot 490,86 \cdot 10^{-3} \text{ kN} \\ &= 127,6 \text{ kN} \end{aligned}$$

$$\sum M_B = 0 \Rightarrow p_u \cdot 3a = N_u \cdot \sin 45^\circ \cdot d$$

$$p_u = \frac{127,6 \cdot \frac{1}{\sqrt{2}} \cdot 4,092}{3 \cdot 1,5} = 82,065 \text{ kN}$$

seguridad $\gamma = \frac{p_u}{p} = \frac{82,06}{50} = 1,64$

en este caso simplemente se agota el rango elástico ya que es una estructura estática.



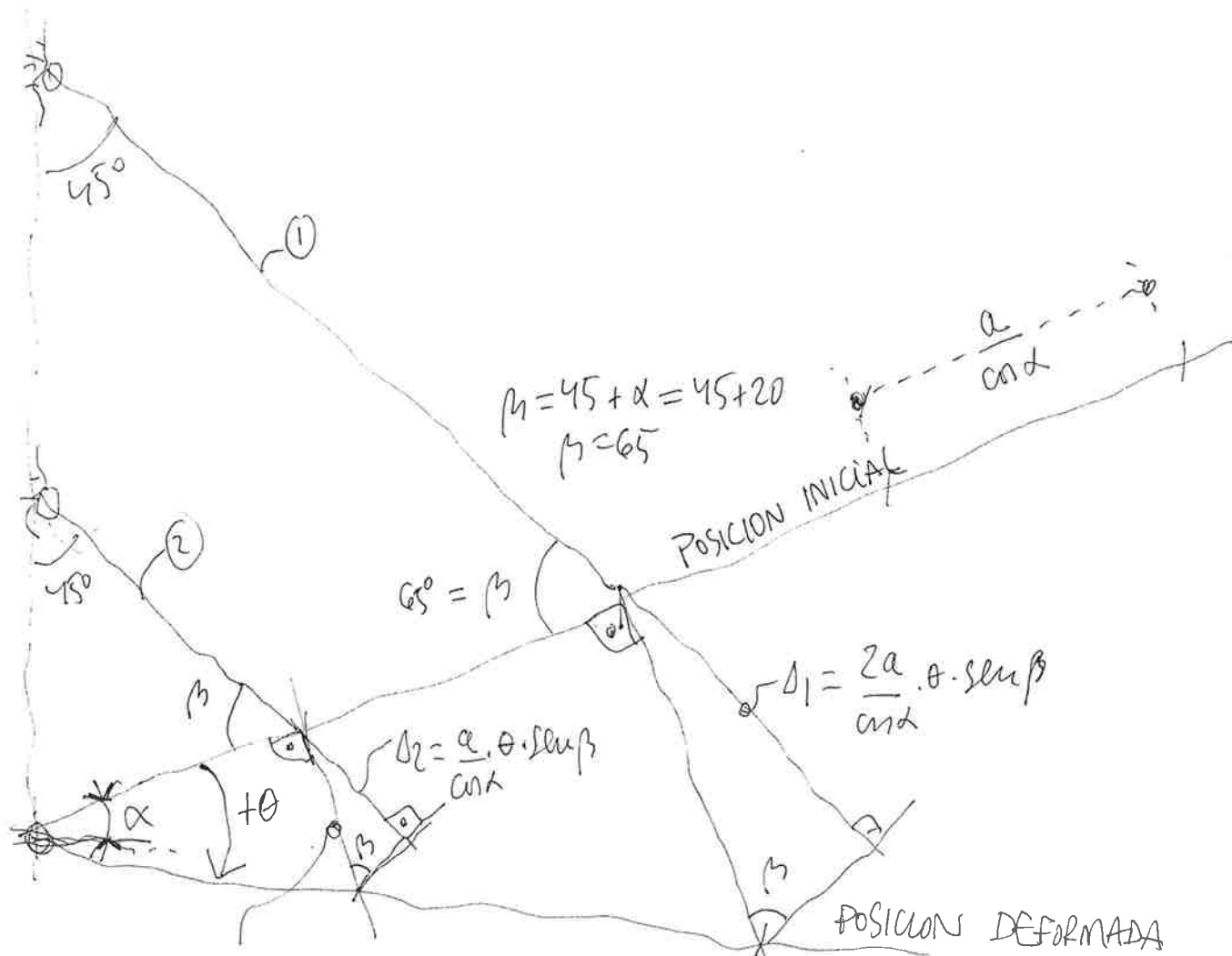
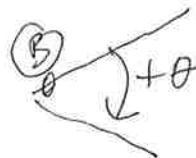
$$\Rightarrow p_u = p \cdot \frac{260}{158,4} = 50 \cdot \frac{260}{158,4} = 82,07 \text{ kN}$$

Se aține un cablu

50
↓

forța de la

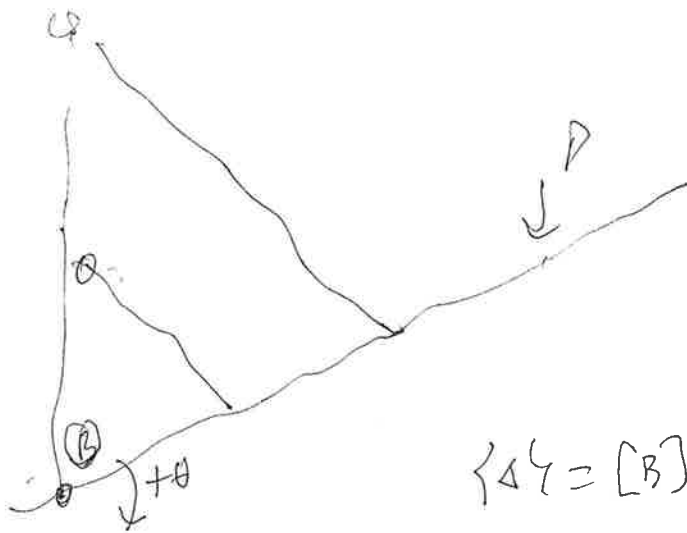
Р.Г. ... г.м. в д.а.м. 3



$$\frac{d \cdot \theta}{\cos \alpha}$$

$$\begin{bmatrix} \Delta_1 \\ \Delta_2 \end{bmatrix} = \begin{bmatrix} \frac{2a \cdot \sin \beta}{\cos \alpha} \\ \frac{a \cdot \sin \beta}{\cos \alpha} \end{bmatrix} \cdot \theta$$

$[B]$ = matrice de comp.



vetor $\{P\}$ associado
a $\{\delta\} = \theta$

$$\{P\} = M_B = P \cdot 3 \cdot a$$

$$\{\Delta\} = [B] \cdot \{\delta\}$$

$$\{P\} = [B]^T \cdot [K_M] \cdot [B] \cdot \{\delta\}$$

$$\underbrace{\quad}_{[K]}$$

$$[K_M] = \begin{bmatrix} k_1 & 0 \\ 0 & k_2 \end{bmatrix}$$

rigidez de
a estrutura.

$$k_i = \left(\frac{EA}{L}\right)_i \dots \text{rigidez de barra.}$$

$$\underbrace{\begin{bmatrix} [B]^T \\ \vdots \end{bmatrix}}_{1 \times 2} \underbrace{\begin{bmatrix} k_1 & 0 \\ 0 & k_2 \end{bmatrix}}_{2 \times 2} \underbrace{\begin{bmatrix} \delta \\ \vdots \end{bmatrix}}_{2 \times 1} = \underbrace{k_1 \cdot \left(\frac{2a}{\cos \beta}\right)^2 + k_2 \cdot \left(\frac{a}{\cos \beta}\right)^2}_{1 \times 1} = [K]$$

$$\{P\} = [K] \cdot \{\delta\} \Rightarrow P \cdot 3 \cdot a = [K] \cdot \theta$$

$$\theta = \frac{P \cdot 3 \cdot a}{[K]} = \frac{50 \cdot 10^3 \cdot 3 \cdot 1,5 \cdot 10^3}{\underbrace{\frac{200 \cdot 10^3}{4,24 \cdot 10^3} \cdot \frac{\pi \cdot 25^2}{4}}_{k_1} + \underbrace{\frac{200 \cdot 10^3}{2,12 \cdot 10^3} \cdot \frac{\pi \cdot 25^2}{4}}_{k_2} \cdot \left(\frac{1,5 \cdot 10^3}{\cos 20} \cdot \sin 65\right)^2}$$

$$\theta = 0,774 \cdot 10^{-3} \text{ rad} \equiv 0,774 \text{ miliradianes} \equiv 0,774 \frac{\text{mm}}{\text{m}} \equiv \text{m.rad}$$

~~+~~ horário

$[B]^T$ es la matriz de equilibrio.

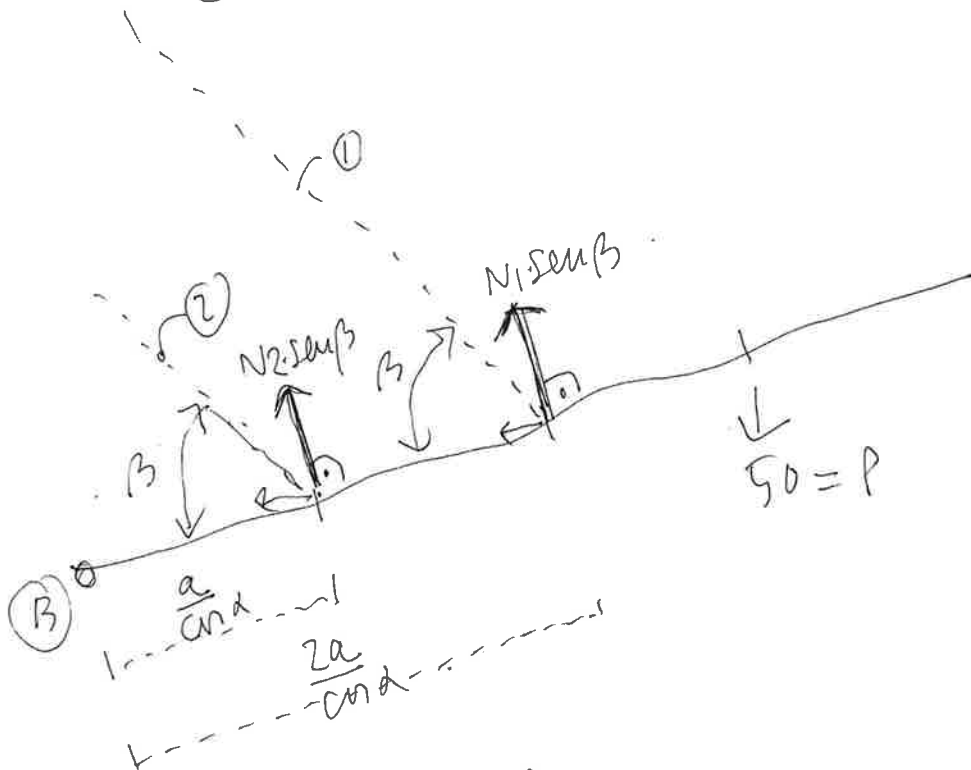
$$\{P\} = [B]^T \cdot \{N\}$$

$$\{P\} = P \cdot 3 \cdot a$$

$$[B]^T = \left[\frac{2a \cdot \sin \beta}{\cos \alpha}, \frac{a \cdot \sin \beta}{\cos \alpha} \right]$$

$$\{N\} = \begin{Bmatrix} N_1 \\ N_2 \end{Bmatrix}$$

¿cómo se deduce esta relación?



$$\sum M(B) = 0$$

$$\Rightarrow N_1 \cdot \sin \beta \cdot \frac{2a}{\cos \alpha} + N_2 \cdot \sin \beta \cdot \frac{a}{\cos \alpha} = \underbrace{P \cdot 3 \cdot a}_{\{P\}}$$

$$\Rightarrow \{P\} = P \cdot 3a = \underbrace{\left[\frac{2a \cdot \sin \beta}{\cos \alpha}, \frac{a \cdot \sin \beta}{\cos \alpha} \right]}_{[B]^T} \cdot \underbrace{\begin{Bmatrix} N_1 \\ N_2 \end{Bmatrix}}_{\{N\}}$$

alargamientos.

$$\{D\} = [B] \cdot \{\delta\} \Rightarrow \begin{bmatrix} \delta_1 \\ \delta_2 \end{bmatrix} = \begin{bmatrix} \frac{2a}{\cos \beta} \cdot \sin \beta \\ \frac{a}{\cos \beta} \cdot \sin \beta \end{bmatrix} \cdot \theta$$

$$\begin{bmatrix} \delta_1 \\ \delta_2 \end{bmatrix} = \begin{bmatrix} \frac{2 \cdot 15 \cdot 10^3 \cdot \sin 65}{\cos 20} \\ \frac{15 \cdot 10^3 \cdot \sin 65}{\cos 20} \end{bmatrix} \cdot \underbrace{0,777 \cdot 10^{-3}}_{\text{rad}} = \begin{bmatrix} 2,24 \\ 1,12 \end{bmatrix} \text{ mm}$$

Axiles o normales.

$$\begin{bmatrix} N_1 \\ N_2 \end{bmatrix} = \begin{bmatrix} k_1 & \\ & k_2 \end{bmatrix} \cdot \begin{bmatrix} \delta_1 \\ \delta_2 \end{bmatrix} = \begin{bmatrix} 23153,7 & 0 \\ 0 & 46307,5 \end{bmatrix} \begin{bmatrix} 2,24 \\ 1,12 \end{bmatrix} = \begin{bmatrix} 51,86 \cdot 10^3 \\ 51,86 \cdot 10^3 \end{bmatrix} \text{ Newtons.}$$

$\{N\} = [kN] \cdot \{\delta\}$

$$\Rightarrow N_1 = N_2 = \underline{51,86 \text{ kN}}$$

¿me fue útil el número axial?

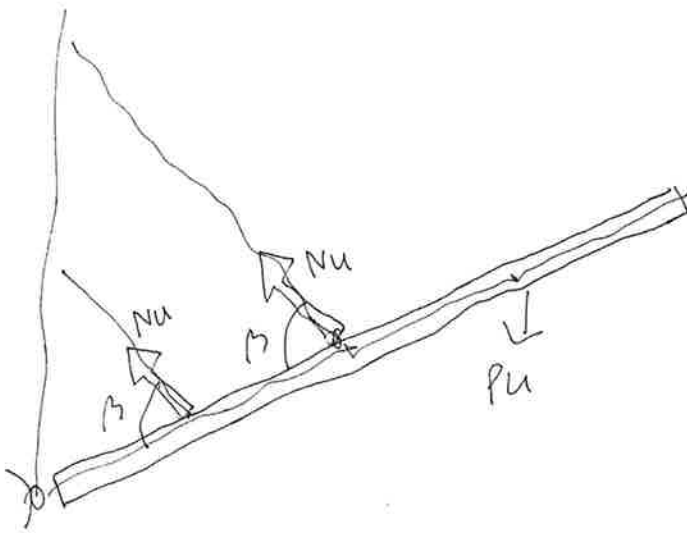
$$\epsilon_i = \frac{\delta_i}{L_i} \Rightarrow \begin{bmatrix} \epsilon_1 \\ \epsilon_2 \end{bmatrix} = \begin{bmatrix} \frac{\delta_1}{L_1} \\ \frac{\delta_2}{L_2} \end{bmatrix} = \begin{bmatrix} \frac{2,24}{4,24 \cdot 10^3} \\ \frac{1,12}{2,12 \cdot 10^3} \end{bmatrix} = \begin{bmatrix} 0,528 \cdot 10^{-3} \\ 0,528 \cdot 10^{-3} \end{bmatrix} \Rightarrow \epsilon_1 = \epsilon_2 = 0,528 \cdot 10^{-3}$$

y además
 $A_1 = A_2$

¡guale áreas!

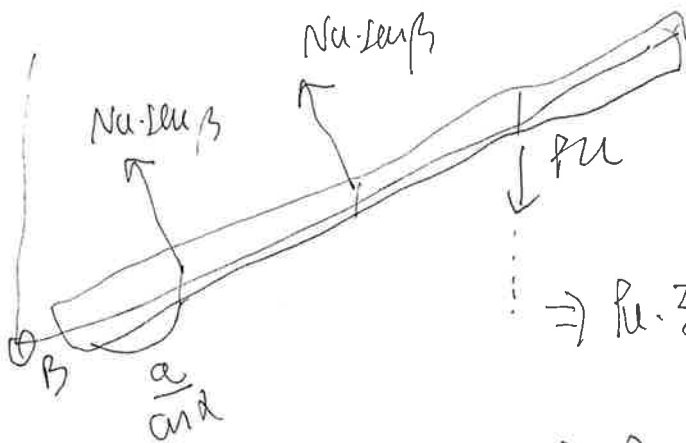
$$\Rightarrow \underline{N_1 = N_2}$$

Carga ultima.



$$\begin{cases} \sigma_c = 260 \text{ N/mm}^2 \\ \phi 25 \end{cases}$$

$$N_u = \sigma_c \cdot A = 127,6 \text{ kN}$$



$$\sum M(B) = 0$$

$$\Rightarrow P_u \cdot 3a = N_u \sin \beta \left(\frac{a}{\cos \alpha} + \frac{2a}{\cos \alpha} \right)$$

$$\Rightarrow P_u = 123,1 \text{ kN}$$

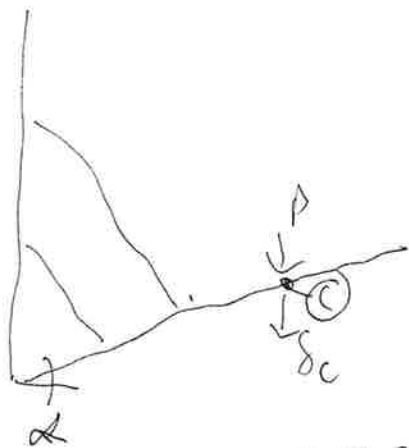
seguridad

$$\gamma = \frac{P_u}{P} = \frac{123,1}{50} = 2,46$$

mayor que en el caso Bntático.

consideremos otro g.d.e.

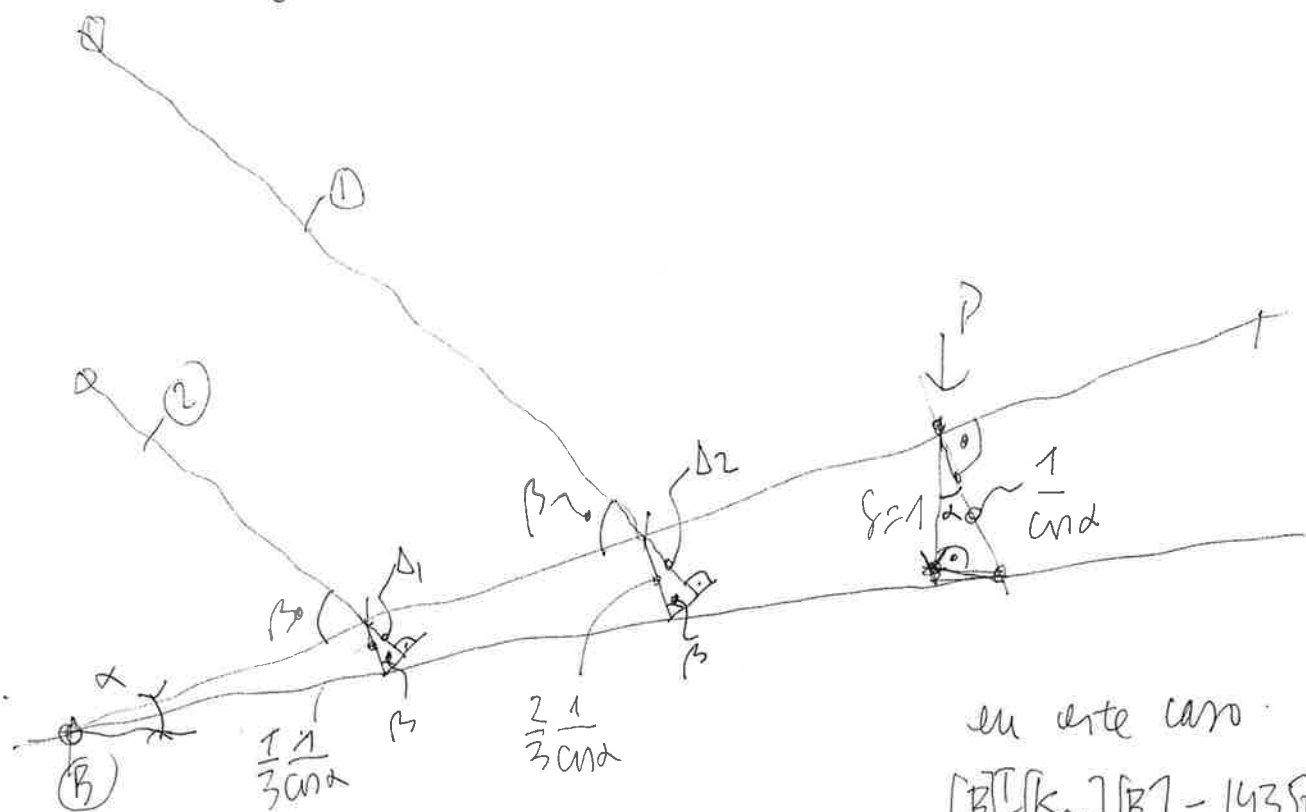
el más sencillo es tomar como $\{f\}$
el descenso en el punto de aplicación de P



$$\{f\} = \delta_c$$

$$\{f\} = P$$

¿cómo es $[B]$?



en este caso

$$[B]^T [K_m] [B] = 14350 \frac{N}{mm}$$

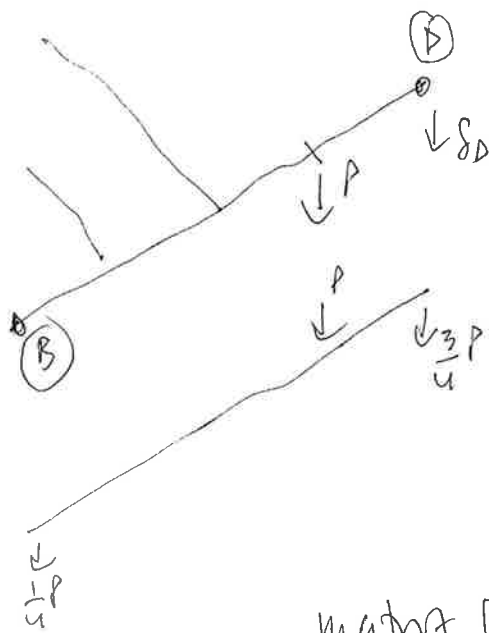
$$\delta_c = \frac{P}{K} = \frac{50 \cdot 10^3}{14350} = 3.48 mm$$

$$\begin{Bmatrix} \delta_1 \\ \delta_2 \end{Bmatrix} = \begin{Bmatrix} \frac{2}{3} \frac{1}{3} \sin \beta \\ \frac{1}{3} \sin \beta \end{Bmatrix} \cdot \delta_c$$

$$\{ \delta \} = [B] \cdot \{ f \}$$

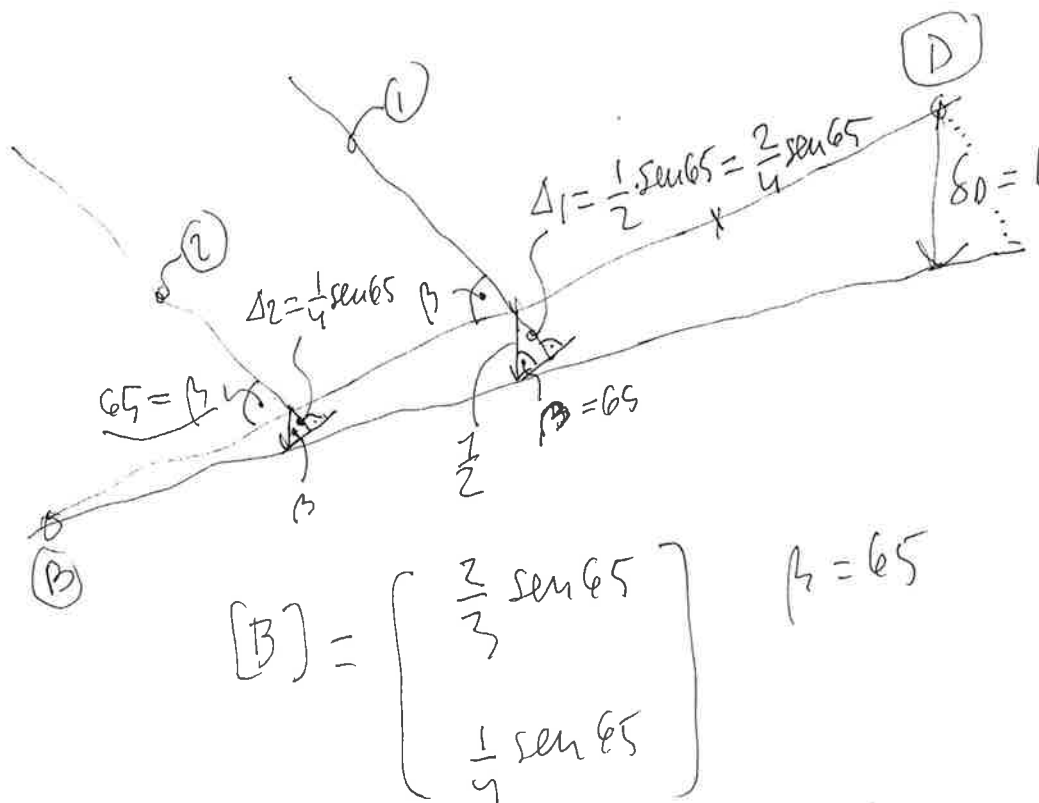
$$\Rightarrow \begin{Bmatrix} \delta_1 = 2.24 \\ \delta_2 = 1.12 \end{Bmatrix} mm$$

Si se usa el descenso en el extremo ya no es tan difícil



$$\Rightarrow \begin{cases} \delta_C = \delta_D \\ 2P\delta = \frac{3P}{4} \end{cases}$$

matriz $[B]$.



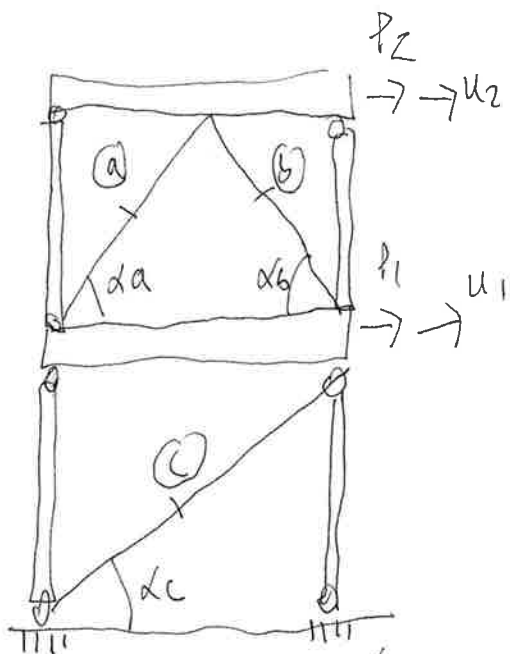
$$[B] = \begin{bmatrix} \frac{2}{3} \sin 65 \\ \frac{1}{4} \sin 65 \end{bmatrix} \quad \theta = 65$$

$$[K] = [B]^T \cdot [K_1] \cdot [B] = \frac{11380}{8071,88} \frac{N}{mm}$$

$$\{P\} = [K] \cdot \{\delta\} \Rightarrow P = [K] \cdot \delta_D$$

$$\delta_D = \frac{P}{[K]} = \frac{50 \cdot 10^3}{\frac{11380}{8071,88}} = \frac{4,645}{17} mm$$

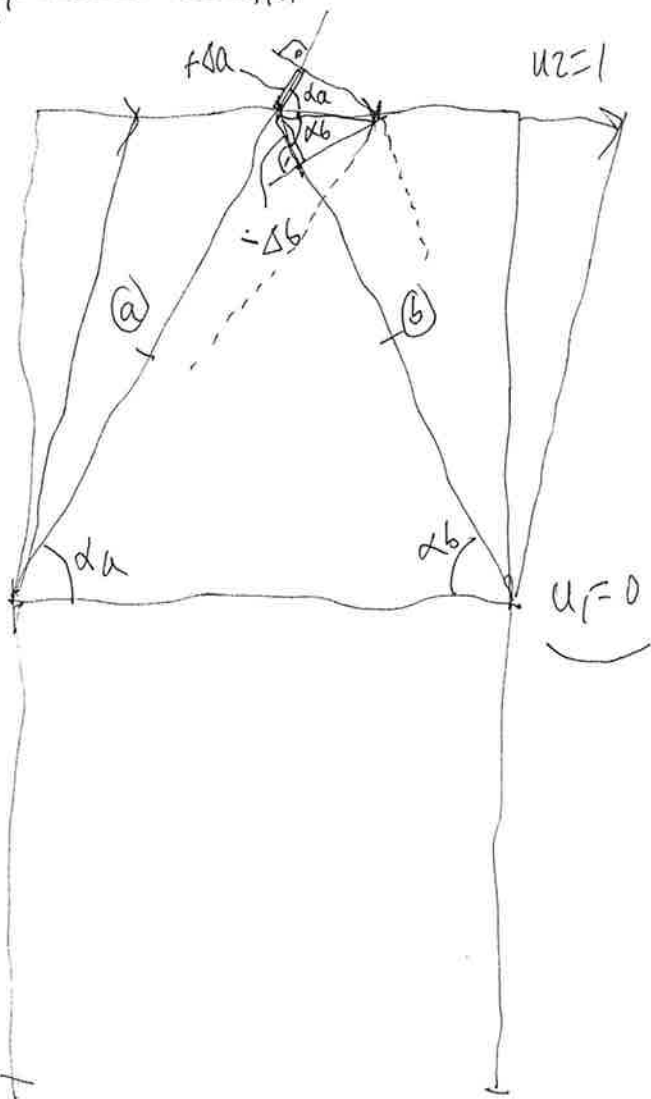
$$\Rightarrow \{\delta\} = [B] \cdot \delta_D \Rightarrow \begin{cases} \delta_1 = 2,24 \\ \delta_2 = 1,12 \end{cases} mm \quad OK, igual$$



(a), (b) y (c) barras deformables, biarticuladas
el resto indeformable.

$$\{P\} = \begin{Bmatrix} P_1 \\ P_2 \end{Bmatrix} \quad \{u\} = \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix}$$

2. g. d. l.



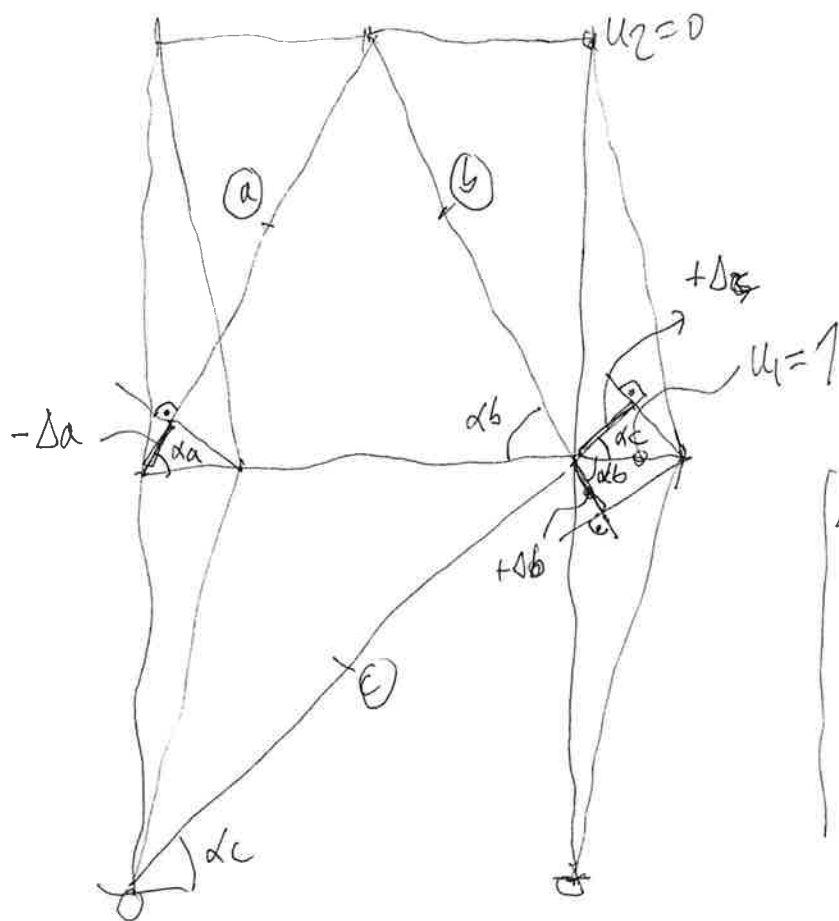
$$+\Delta a = u_2 \cdot \cos \alpha_a$$

$$-\Delta b = u_1 \cdot \cos \alpha_b$$

segunda columna de [B]

$$\Rightarrow \begin{Bmatrix} \Delta a \\ \Delta b \\ \Delta c \end{Bmatrix} = \begin{Bmatrix} \cos \alpha_a \\ -\cos \alpha_b \\ 0 \end{Bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix}$$

$$\{\Delta\} = [B] \cdot \{u\}$$



$$-\Delta a = u_1 \cdot \cos \alpha_a$$

$$+\Delta b = u_1 \cdot \cos \alpha_b$$

$$+\Delta c = u_1 \cdot \cos \alpha_c$$

$$\begin{Bmatrix} \Delta_1 \\ \Delta_2 \\ \Delta_3 \end{Bmatrix} = \begin{Bmatrix} -\cos \alpha_a \\ \cos \alpha_b \\ \cos \alpha_c \end{Bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix}$$

Primera columna
[B]

es decir, considerando ambos casos juntos

$$[B] = \begin{bmatrix} -\cos \alpha_a & \cos \alpha_a \\ \cos \alpha_b & -\cos \alpha_b \\ \cos \alpha_c & 0 \end{bmatrix}$$

Soludo además

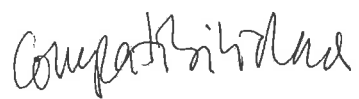
$$[k_m] = \begin{bmatrix} k_a & 0 & 0 \\ 0 & k_b & 0 \\ 0 & 0 & k_c \end{bmatrix}$$

$$k_i = \left(\frac{EA}{L} \right)_i \text{; rigidez de barra}$$

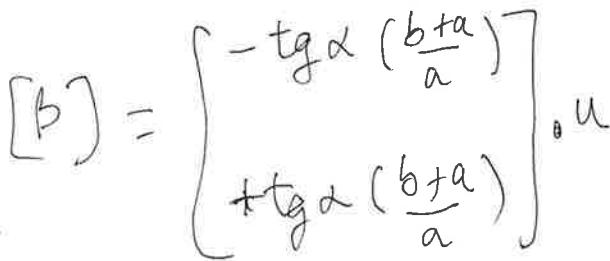
$$[K] = [B]^T \cdot [k_m] \cdot [B]$$

$$\left\{ \begin{matrix} P_1 \\ P_2 \end{matrix} \right\} = \left\{ \begin{matrix} u_1 \\ u_2 \end{matrix} \right\} \left\{ \begin{matrix} P_1 \\ P_2 \end{matrix} \right\} = [B]^T \cdot [k_m] \cdot [B] \cdot \left\{ \begin{matrix} u_1 \\ u_2 \end{matrix} \right\}$$

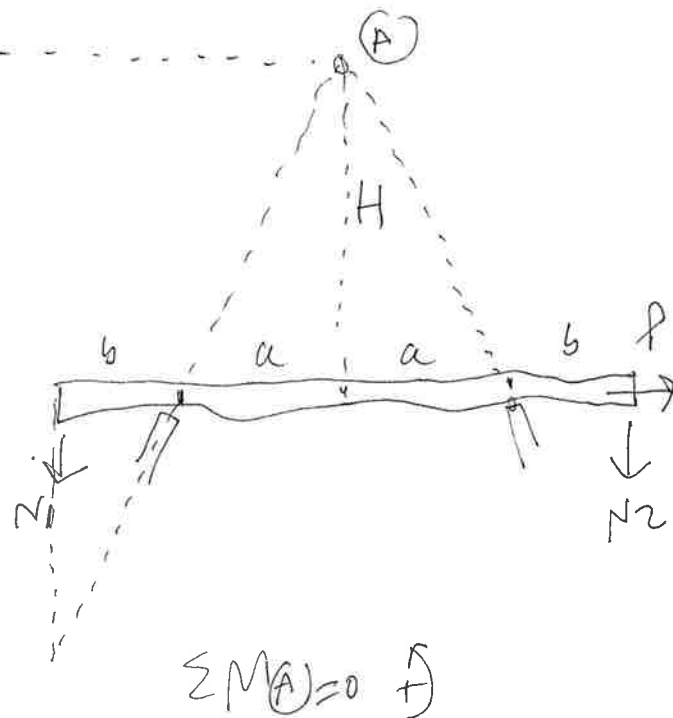
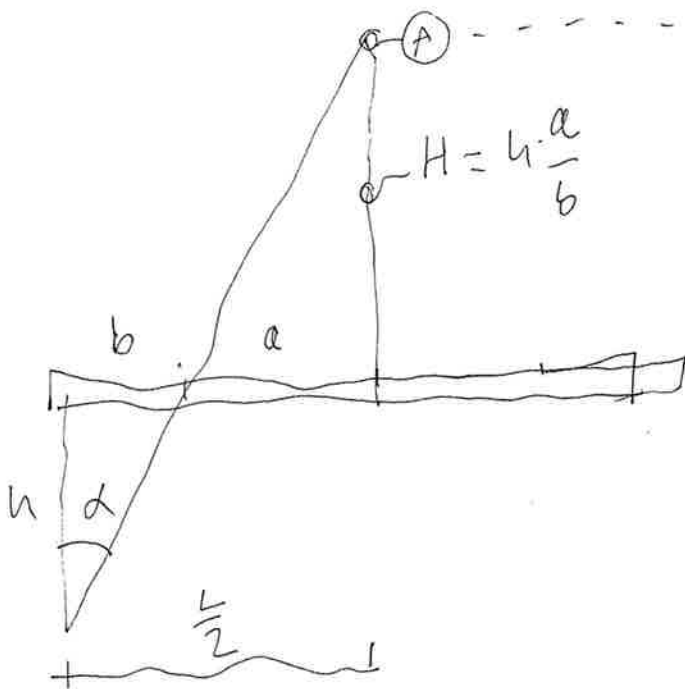
[K] = rigidez de estructura.



389 = u ... 1 g.d.l.



¿que situación de equilibrio conduce a una $[B]^T$ asociada a la anterior.



$$P \cdot H + N_1 \cdot \frac{L}{2} - N_2 \cdot \frac{L}{2} = 0$$

$$P = -N_1 \cdot \frac{L}{2H} + N_2 \cdot \frac{L}{2H}$$

$$\{P\} = P \quad \{N\} = \begin{bmatrix} N_1 \\ N_2 \end{bmatrix} \Rightarrow P = \left[-\frac{L}{2H}, \frac{L}{2H} \right] \begin{bmatrix} N_1 \\ N_2 \end{bmatrix}$$

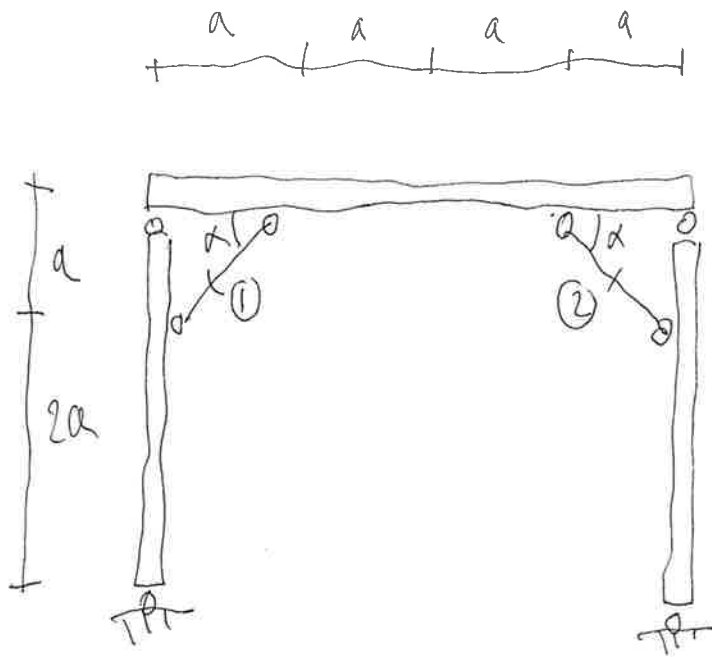
$$\{P\} = [B]^T \cdot \{N\}$$

pero reorganizando $[B]^T$.

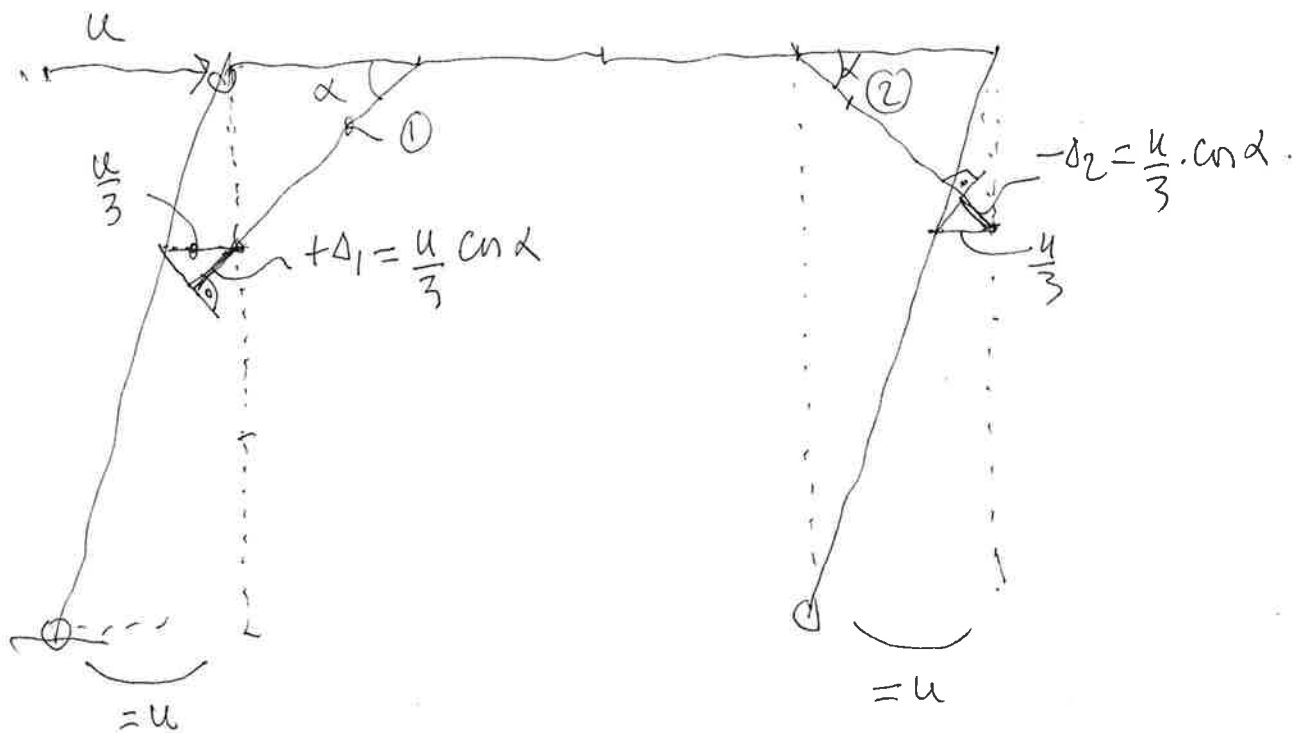
$$\left. \begin{array}{l} \frac{L}{2} = a + b \\ H = h \cdot \frac{a}{b} \end{array} \right\} \Rightarrow \frac{L}{2} \frac{1}{H} = (a + b) \cdot \frac{b}{a \cdot h} = \operatorname{tg} \alpha \cdot \frac{(a + b)}{a}$$

$$\Rightarrow [B]^T = \left[-\operatorname{tg} \alpha \cdot \frac{(a + b)}{a}, +\operatorname{tg} \alpha \cdot \frac{(a + b)}{a} \right]$$

como antes.



① y ② deformable,
el resto indeformable.

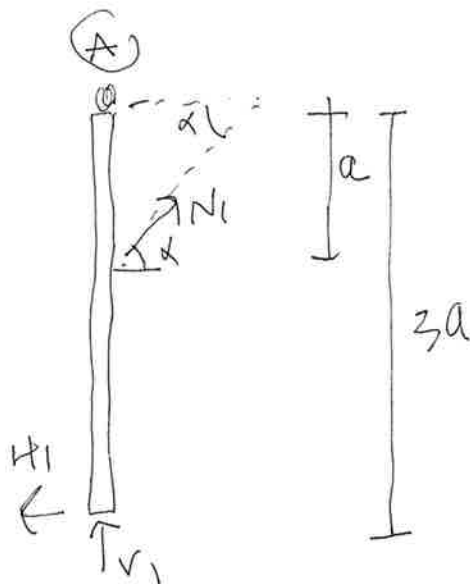
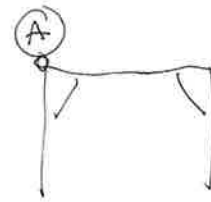


$$\begin{vmatrix} \Delta_1 \\ \Delta_2 \end{vmatrix} = \begin{vmatrix} \frac{1}{3} \cos \alpha \\ -\frac{1}{3} \cos \alpha \end{vmatrix} \cdot u$$

$$\{\Delta\} = [B] \cdot \{84\}$$

obtener $\begin{vmatrix} N_1 \\ N_2 \end{vmatrix} \dots$

por equilibrio de soporte --

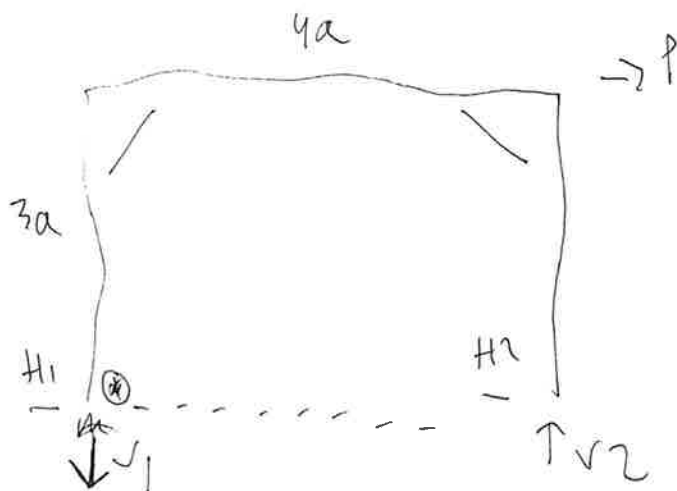


$$\sum M_A = 0$$

$$N_1 \cdot \sin \alpha \cdot a = H_1 \cdot 3a$$

$$H_1 = \frac{N_1 \cdot \sin \alpha}{3}$$

... de forma similar H_2 --



V_1 y V_2 salen
por equilibrio global
directamente
(sin depender de los)
enquises

$$\Rightarrow \left. \begin{aligned} P \cdot 3a &= V_2 \cdot 4a \\ P \cdot 3a &= V_1 \cdot 4a \end{aligned} \right\} V_1 = V_2 = \frac{P \cdot 3a}{4a} = \frac{3}{4} P$$

... con el sentido indicado